## A New Moving－Solid Algorithm for Landslide Tsunami and Boulder Movement

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## ABSTRACT

In this presentation, we will present our recent research results on the potential tsunami hazard in the South China Sea and the Philippine Sea. The methodology in determining the fault parameters, such as the fault plane area, length, width, dislocation, and asperity, along the Manila subduction zone is first discussed. Numerical simulations of tsunami generation, propagation and inundation, based on several scenario earthquakes, are conducted to assess potential tsunami hazard in the region. Our results show that the potential tsunamis, generated along the Manila Trench, would mainly affect the west coast of Luzon Island, the East coast of Vietnam, the South of Taiwan, Hong Kong, and the south coast of Mainland China.
For the purpose of establishing a tsunami warning system in the region, a numerical algorithm is also developed to determine the most effective locations for deploying deep ocean pressure sensors. Finally, a newly developed Impact Intensity Analysis (IIA) method will be presented. This method is used to identify the locations of tsunami source that could generate tsunamis affecting the study site. One of the important applications of the IIA method is for mitigating tsunami hazard affecting coastal nuclear power plants. It is also a useful tool for locating the tsunami source of historical and paleo tsunamis. Validation and demonstration of the IIA method will be presented for the 1867 Keelung tsunami event in Taiwan, since 3 nuclear power plants are located nearby.



Tsunami Source
Characterization for Western Pacific Subduction Zones: A Preliminary Report
USGS1 Tsunami Subduction
Source Working Group
BOTTOM LINE Hazard appraisal key:
A: High
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Recently the USGS issued a report assessing the potential risk as a tsunami source along the entire Pacific seduction zones. One highly risk zone is identified along the Manila (Luzon) trench, where the Eurasian plate is actively subducting eastward underneath the Luzon volcanic arc on the Philippine Sea plate.

## Estimation of Return period



Source:ANSS 1963-2006


It is significant that since the Spanish colonization of Luzon in the 1560s, no earthquake exceeding magnitude 7.8 has been observed (Repetti, 1946). Conservatively, it can be postulated that very large events on this Megathrust have a recurrence interval exceeding 440 years. Taking a trench-normal convergence velocity of $87 \mathrm{~mm} / \mathrm{yr}$, strain of $\sim 38 \mathrm{~m}$ would range of plausible scenarios. It is comparable to the 1960 Mw 9.5 Chilean earthquake, in which coseismic slip reached 40 m (Barrientos and Ward, 1990), and larger than 2004 Aceh-Andaman event, which produced 20 m of coseismic slip (Chlieh et al., 2007).


The sinuous rupture interface of the South China Sea megathrust, together with ten seismic cross sections between latitude 12.5 N and 23.5 N from the studies by Bautista et al. (2001) and Wu et al. (2007). Epicenters of thrust-faulting earthquakes are plotted to mark the downdip boundary of the rupture interface.


GPS data (Yu et al., 1999) indicating motion of the converging Eurasian Plate and the Philippines Sea Plate, where the blue arrows and numbers show raw velocity values ( $\mathrm{mm} / \mathrm{yr}$ ) taken from Yu et al. (1999), the red arrow and numbers indicate velocity values ( $\mathrm{mm} / \mathrm{yr}$ ) resolved in the direction perpendicular to the trench front, and the black numbers give the rounded values ( $\mathrm{mm} / \mathrm{yr}$ ) used for slip estimation.
(Megawati et at., 2009)




Can we find the hot spots for the study site systematically?

## News Report：


星期四 2014年08月
14日


| tr | 大陸 | 港溶台 | 國際 | 財經 | 嚯點 | 圖解 | 南早郞港指南 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 首頁 $n$ 國際 $n$ 馬尼拉海溝地震風險加劇 南海爭端阻預測工作 |  |  |  |  |  |  | 熱門話題：中國反鹖 | ＂福喜肉＂事件 |

－国際
馬尼拉海溝地震風險加劇南海爭端阻預測工作


－電郵
2014年08月06日下午1：05
中國大陸，台灣和靠律貝的科學家都認為，南海周邊國家低估了南海海域發生大海啸的風險。

一旦海啸發生，包括香港在內的沿海地區可能會有數蕅人喜生，導致損失慘重。

中國科學院海洋研究所一位科學家稱，他們覀需最新數捷來評估海啸可能爆發的規模和時間，但由於南海主權糾紛，他們無法前赴該區域獲取數據。

## Tsunami Model: COMCOT <br> (Cornell Multi-grid Coupled Tsunami Model)



- Capable of simulating the entire lifespan of a tsunami, from its

COMCOT: A Tsunami Modeling Package


Tsunami Generation


Tsunami Propagation

COMCOT (Cornell Multi-grid Coupled Tsunami Model) is a tsunami modeling package, capable of simulating the entire lifespan of a tsunami, from its generation, propagation and runup/rundown in coastal regions.

## - Governing Equations

COMCOT was developed based on Shallow Water Equations (SWE) in Spherical Coordinates (Eq.01) and Cartesian Coordinates (Eq.02). In the equations, $\zeta$ denotes free surface elevation; $P$ and $Q$ are volume flux in $x$ and $y$ direction $(P=h u, Q=h v) ; \varphi$ and $\psi$ stand for longitude and latitude, respectively.

$$
\begin{array}{ll}
\frac{\partial \zeta}{\partial t}+\frac{1}{R \cos \varphi}\left[\frac{\partial P}{\partial \psi}+\frac{\partial}{\partial \varphi}(\cos \varphi Q)\right]=0 & \frac{\partial \zeta}{\partial t}+\frac{\partial P}{\partial x}+\frac{\partial Q}{\partial y}=0 \\
\frac{\partial P}{\partial t}+\frac{g h}{R \cos \varphi} \frac{\partial \zeta}{\partial \psi}-f Q=0 & \frac{\partial P}{\partial t}+\frac{\partial}{\partial x}\left(\frac{P^{2}}{H}\right)+\frac{\partial}{\partial y}\left(\frac{P Q}{H}\right)+g H \frac{\partial \zeta}{\partial x}+\frac{\tau_{x} H}{\rho}=0 \\
\frac{\partial Q}{\partial t}+\frac{g h}{R} \frac{\partial \zeta}{\partial \varphi}+f P=0 & \frac{\partial Q}{\partial t}+\frac{\partial}{\partial x}\left(\frac{P Q}{H}\right)+\frac{\partial}{\partial y}\left(\frac{Q^{2}}{H}\right)+g H \frac{\partial \zeta}{\partial y}+\frac{\tau_{y} H}{\rho}=0
\end{array}
$$

generation, propagation and runup/rundown on coastal regions

- A numerical model which solves nonlinear shallow water equation (SWE).
- On both/either Spherical or Cartesian coordinate system.
- Using nested grid to solve multi-scale problems.
- Moving-boundary for inundation calculation
- Parallelized
- THANKS, Dr. Xiaoming WANG


## - Moving Boundary Scheme

Moving boundary scheme was also introduced in COMCOT to model the run-up and run-down. The instant "shoreline" is defined as the interface between a dry grid and wet grid and volume flux normal to the interface is assigned to zero.


2011 Tōhoku earthquake and tsunami

- We spent about 20 minutes to prepare, or wait for, the fault parameters
- COMCOT spent about 1 min to finish the tsunami simulation from Japan to Taiwan.
- It is about real-time simulation
- COMCOT predicted that the tsunami wave height was about 12 cm offshore Taiwan.
- Field data also showed 12 cm .


## Initial Free－Surface of 311 Japan Tsunami Event



模式預測海床抬昇量為4．5公尺，與實際觀測之5公尺相當接近。

## Animation of Tsunami Propagation



模式預測之海嘯波高與日本潮位站實測比對：Hanasaki



Hanasaki 潮位站比對，藍線為模擬結果，黑線為實測資料。該站位於斜坡部分，模擬結果與實測比對相當一致。

## 模式預測之海嘯波高中央氣象局潮位站資料比對

Hualien


台灣測站比對。比對花蓮，東港，小琉球，蘭嶼四個測站，結果相當理想。（藍線為模擬結果，黑線為實測資料，資料提供：中央氣象局）

## iCOMCOT：

## a grid／cloud－based Tsunami system



在中研院網格中心協助下，將COMCOT模式提昇為雲端系統，以利其他國家之海嘯災防
wrunul suaface
intial surface


Initial Free－Surface Elevation

ICOMCOT Hene Aboat Smulation－Status Contact Logat $\mathbf{2}$ scstwssemo



# (1) 2012 Invited Speech at UNESCO (2) Interviewed by isgtw, London, UK 

## http://www.isgtw.org/feature/forecasting-wrath-tsunami

## isgtw <br> international science grid this week

## Forecasting the wrath of a tsunami

=EATURE | APRIL 24, 2013 | BY ZARA QADIR
mmediately to the south-west of raiwan, is the South China Sea and :he deep oceanic Manila trench. zoughly every 10 years, the area experiences a moderate earthquake under 6.9 on the Richter scale). towever, there has not been a najor earthquake since the 1570 s. 3PS data and global historical ecords show that every 700 years an earthquake of magnitude 9.0 is an earthquake of magnitude 9.0 is ikely to strike the area. The region, herefore, is due one relatively soon in terms of geological time frames) and if (or when) a mega-sized o



The spatial distribution of 18 trench-typed tsunami sources (T1-T-18) and 4 fault-typed tsunami sources (T19-T-22). The color bar indicates the seafloor displacement of each tsunami source. Click for large version. Image courtesy Simon Lin, ASGC, from Tso-Ren Wu's paper.

COMCOT (Cornell Multi-grid Coupled Tsunami Model) is a numerical model that allows both simulation and visualization of the whole lifespan of a tsunami. It shows how a wave will travel on the earth and gives an estimate of its arrival time and the level of run up on to dry land. "The original research model focuses on accuracy and not speed; it took between 12 to 24 hours to generate a result. But for the system to be operational, COMCOT needed to simulate a tsunami as fast as real time propagation, from hours to minutes," says Wu.

Usually an operational system sacrifices some level of accuracy, but COMCOT allows both linear and non-linear equations. "A linear system speeds up the operation and is accurate for the deep ocean, but is not precise enough for the near-shore region. When the wave approaches the shoreline its speed diminishes, and it becomes thinner and taller so the curve can no longer be represented linearly. Most tsunami systems ignore this part of the simulation but it is the most important to impact on human life," explains Wu. COMCOT integrates the spherical with a Cartesian coordinate system, which is more accurate for near shore simulations.



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Tsunami Sources of
18 Trench and 4 Fault Segments
18 Trench-type tsunami sources ( $\mathrm{T} 1 \sim \mathrm{~T} 18$ ) 4 Fault-type tsunami sources ( $\mathrm{T} 19 \sim \mathrm{~T} 22$ )


## Nested Grids



Layer 1: 2 min ( $\sim 3500 \mathrm{~m}$ );
Layer 2: $1 / 2 \mathrm{~min}(\sim 900 \mathrm{~m})$;
Layer 3: $1 / 8 \mathrm{~min}(\sim 200 \mathrm{~m})$;
Layer 4: 1/128 min (~50m);
Layer 5: $1 / 512 \mathrm{~min}(\sim 10 \mathrm{~m})$;
Layer 6: $1 / 2048 \mathrm{~min}(\sim 2 \mathrm{~m})$;


## Source of Bathymetry

- ETOTO2: (2 arc min)
- http://www.ngdc.noaa.gov/mgg/gdas/gd_designagrid. htm '
- GEBCO: (0.5 arc min)
- http://www.gebco.net/data_and_products/gridded_ba thymetry_data/ ${ }^{\circ}$
- NAVY
- NCU: 40m DEM
- National Land Surveying and Mapping Center: 10m DEM
- Tai Power: 1m DEM


## Tsunami Sources of 18 Trench Segments

Taiwan has to be aware of the tsunamis from $\mathrm{T} 1, \mathrm{~T} 2, \mathrm{~T} 3$, and T 8


## T2 (Manila Trench 1) (Animation)



## T02, Inundation and Maximum Runup Height



## T02, Nearshore Inundation and Maximum Runup Height



Runup height: NP3: 10~12m; South Bay: 18m

The First National-wide Tsunami Drill in Taiwan in 2014/9/19


## （畨台深電力公司




1．震央經度
2．震央緯度
3．震源深度
4．地震規模

1．震央經度
2．震央緯度
3．震源深度
4．地震規模
5．Strike
6．Dip
7．Slip
1．震央經度
2．震央緯度
3．震源深度
4．地震規模
5．Strike
6．Dip
7．Slip
8．斷層破裂長寬
9．錯動量
10．其他模擬設定

## Tsunami Fast Calculation System for CWB in Taiwan．

Fully automatic．One－click to finish them all．

Simulation with the region covers the PS and SCS can be done in 1.5 mins．

COMCOT計算 \＆繪圖




## Database for earthquake parameters海溝走向資料庫一提升準確性

資料庫參考過去學者針對台灣具有潛在海嘯威脅之海溝所訂定之參數 （ $\mathrm{Wu}, 2012$ ）所設計。若地震位於資料庫外，則走向平行於台灣海岸線，判斷方式為，以南投虎子山一等三角點代表台灣中心，走向垂直於震源和虎仔山連線方向。


| 海溝名稱 | 走向（度） | 適用經度範園 （ ${ }^{\circ}$ E） | 適用緯度範園 （ ${ }^{\circ} \mathrm{N}$ ） |
| :---: | :---: | :---: | :---: |
| T1花蓮外海 | －66．2422 | 121.9 ～123．5 | $23.8 \sim 24.5$ |
| T2馬尼拉海清1 | 340.7619 | 119．25～120．75 | $19.5 \sim 22.0$ |
| T3馬尼拉海溝2 | 35.3532 | $119.0 \sim 121.0$ | $17.5 \sim 20.0$ |
| T4馬尼拉海清3 | 2.403 | 118．5～120．5 | 13．5～19．0 |
| T5馬尼拉海溝4 | 313.0466 | $119.0 \sim 121.0$ | $12.5 \sim 14.0$ |
| T6菲律實海清1 | 328.3928 | $123.5 \sim 125.5$ | $13.5 \sim 15.5$ |
| T7菲律賓海溝2 | 347.6032 | $125.5 \sim 127.5$ | $5.5 \sim 13.5$ |
| T8亞普海溝 | 44.9191 | $135.0 \sim 140.0$ | $6.0 \sim 12.0$ |
| T9馬里亞納海溝1 | 74.3247 | $141.0 \sim 145.5$ | $10.5 \sim 13.5$ |
| T10馬里亞納海清2 | 24.4308 | $145.5 \sim 150.5$ | $12.5 \sim 17.5$ |
| T11馬里亞納海溝3 | －9．6795 | $146.0 \sim 149.0$ | $16.5 \sim 22.5$ |
| T12馬里亞納海溝4 | －42．1025 | $143.0 \sim 149.0$ | 22．5～25．0 |
| T13伊豆－小栗原海溝1 | －4．1057 | $141.0 \sim 144.0$ | $26.0 \sim 30.0$ |
| T14伊豆－小栗原海溝2 | －10．9672 | $140.0 \sim 144.0$ | $30.0 \sim 35.0$ |
| T15南海海淟 | －115．806 | $132.5 \sim 140.0$ | $31.0 \sim 35.0$ |
| T16琉球島弧1 | －154．62 | $130.0 \sim 132.5$ | $27.5 \sim 31.0$ |
| T17琉球島弧2 | －134．981 | 126．0～130．0 | $23.0 \sim 27.5$ |
| T18琉球島弧3 | －95．1302 | $123.5 \sim 126.0$ | 23．0～24．75 |

海溝之参數與範圍分布

## Japan 日本

## Validation

By assuming that only four parameters were known in the early stage．


Taiwan 台灣


初步参數（present）較詳細參數（GCMT）


Russia 俄羅斯


USA 美國



Hanasaki＿2



較詳細參數（GCMT）

## Landslide and Local Scour

## Motivation

Tsunami Boulders were found in the Southern Taiwan


## Motivation

One of the boulders is in a huge scour hole
The broken coral boulder implies an originally much bigger size and higher tsunami wave height


## The failure of Shuang－Yuan Bridge in the event of 2009 Typhoon Morakot．

The undular waves indicate the soft reverbed and sever local scour around the bridge piles． 2009 莫拉克颱風強烈水流導致雙園橋斷裂
波狀水躍暗示床質鬆軟及橋墩周圍沖刷


## Breaking wave modeling，Splash3D（史百力士3D）

We adopted the Splash3D numerical model to solve for the breaking wave problems（Wu，2004；Liu et al．，2005）．This model solves 3－dimensional incompressible flow with Navier－Stokes equations．The free－surface is tracked by Volume－of－Fluid（VOF）method．The domain is discretized by finite volume method（FVM）．The turbulent effect is closed by large eddy simulation（LES）with Smagorinsky model．

Incompressible continuity equation：
$\nabla \cdot \boldsymbol{u}=0$
Navier－Stokes Equation
$\frac{\partial(\boldsymbol{u})}{\partial t}+\nabla \cdot(\boldsymbol{u} \boldsymbol{u})=-\frac{1}{\rho} \nabla P+\frac{1}{\rho} \nabla \cdot \ell+\boldsymbol{g}+\boldsymbol{F}_{0}$

Splash：飛戝



Disney Splash Mountain迪士尼史百力士山

## Volume of Fluid (VOF) method

The fluid density is presented in fluid fraction, and the transport equation is used to describe the fluid movement.
$\frac{\partial \rho_{m}}{\partial t}+\nabla \cdot\left(\rho_{m} \boldsymbol{u}\right)=\frac{\partial \rho_{m}}{\partial t}+\boldsymbol{u} \frac{\partial \rho_{m}}{\partial x}+\boldsymbol{v} \frac{\partial \rho_{m}}{\partial y}+\boldsymbol{w} \frac{\partial \rho_{m}}{\partial z}=0$
$\rho=\sum_{m} f_{m} \rho_{m}^{0}$
$\frac{\partial f_{m}}{\partial t}+\nabla\left(\boldsymbol{u}_{i} f_{m}\right)=0$
Piecewise linear interface calculation (PLIC)
$\stackrel{\mathrm{v}}{ }{ }^{\prime} \cdot x_{p}-C_{p}=0$

| 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| :---: | :---: | :---: | :---: | :---: |
| 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 0.0 | 0.2 | 0.1 | 0.0 | 0.0 |
| 0.8 | 1.0 | 0.7 | 0.1 | 0.0 |
| 1.0 | 1.0 | 1.0 | 0.6 | 0.0 |
| 1.0 | 1.0 | 1.0 | 1.0 | 0.8 |

$F\left(C_{p}\right)=V_{t r}\left(C_{p}\right)-f_{m} * \forall \approx 0$

## DEM and LiDAR topography input module and COMCOT boundary coupling module



## Partial-Cell treatment

$$
\begin{aligned}
& \forall_{\text {eff }}=\left(1-f_{\text {solid }}\right) \forall=\theta \\
& \partial \frac{\left(\theta f_{m}\right)}{\partial t}+\nabla \cdot\left(\theta f_{m} V\right)=0
\end{aligned}
$$

Topography of Toce River Valle
LiDAR input


## Model Validation 1: Dam-break bore impinging a square cylinder



## Model Validation: Free-surface



Time $=0$




Time $=0.9604936$


Time $=1.300088$



## Grid Setup: <br> Non-uniform in the spam-wise direction



The Detail of the Truss, and the Surface-Force integration cells with normal vectors.


## Wave + Current + Truss Surface Elevation and Dynamic Pressure



## Surface Velocity Magnitude



## Sloshing Problem

2015 Nepal Earthquake, Swimming pool.





## Potential tsunami impact on the Nuclear Power Plant (NPP) No. 3 in Taiwan.

Splash3D Coupled with the result of 2D COMCOT tsunami model
 northern Manila Trench

## Dynamic Two-Way Coupling Implicit Velocity Correction Method (IVCM)

$$
\left.\begin{array}{l}
\frac{\partial \rho}{\partial t}+\nabla \cdot(\rho \mathbf{u})=0 \\
\frac{\partial(\rho \mathbf{u})}{\partial t}+\nabla \cdot(\rho \mathbf{u u})=-\nabla P+\nabla \cdot \tilde{\tau}+\mathbf{f}_{B} \\
\frac{\rho^{n+1} \mathbf{u}^{*}-\rho^{n+1} \mathbf{u}^{*}}{\Delta t}=-\nabla \cdot(\rho \mathbf{u u})^{n}-\nabla P^{n}+\nabla \cdot\left(\mu^{n+1}\left(\nabla \mathbf{u}+\nabla^{\mathrm{T}} \mathbf{u}\right)\right)+\mathbf{f}_{B}^{n} \\
\frac{\rho^{n+1} \mathbf{u}^{* *}-\rho^{n+1} \mathbf{u}^{*}}{\Delta t}=-\nabla \delta P^{* * *}+\mathbf{f}_{B}^{n+1}-\mathbf{f}_{B}^{n} \\
\nabla \cdot \frac{\nabla \delta P^{* *}}{\rho^{n+1}}=\nabla \cdot\left(\frac{\mathbf{u}^{*}}{\Delta t}+\frac{\mathbf{f}_{B}^{n+1}}{\rho^{n+1}}-\frac{\mathbf{f}_{B}^{n}}{\rho^{n+1}}\right) \\
\mathbf{u}^{* *}=\mathbf{u}^{*}-\Delta t\left(\frac{\nabla \delta P^{* * *}-\mathbf{f}_{B}^{n+1}+\mathbf{f}_{B}^{n}}{\rho^{n+1}}\right) \\
\text { Loop until }\left(\mathrm{abs}\left(\mathbf{u}^{n+1}-\mathbf{u}_{s}\right)<\text { tolerance }\right) \\
\mathbf{f}_{s}^{n+1}=\rho^{n+1}\left(\frac{\mathbf{u}_{s}^{n+1}-\mathbf{u}^{* *}}{\Delta t}\right) \\
\frac{\rho^{n+1} \mathbf{u}^{n+1}-\rho^{n+1} \mathbf{u}}{} \mathbf{u}^{* *} \\
\Delta t \\
\nabla \cdot \nabla \delta P^{n+1}+\mathbf{f}_{B}^{n+1}-\mathbf{f}_{B}^{n}+\mathbf{f}_{s}^{n+1} \\
\rho^{n+1}=\nabla P^{n+1} \\
\mathbf{u}^{n+1}=\mathbf{u}^{* *}-\Delta t\left(\frac{\mathbf{u}^{* *}}{\Delta t}+\frac{\mathbf{f}_{B}^{n+1}}{\rho^{n+1}}-\frac{\mathbf{f}_{B}^{n}}{\rho^{n+1}}+\frac{\mathbf{f}_{s}^{n+1}}{\rho^{n+1}}\right) \\
\rho^{n+1}
\end{array}\right)
$$

## Floating Obstacle

Numerical setup of the floating bodies.


The photos with dimension of the small tank (upper left) and the large tank (upper right). The floating box is made of wood (lower right). A small black dot is painted on it to trace the floating trajectory. The still water depth is 5 cm . The box is initially elevated 0.2 cm by four pins. (Lower left).

## Simulation on a Floating Obstacle



（莊美惠製）

- 渠槽大小： $15 \times 14 \mathrm{~cm}, ~ 30 \times 30 \mathrm{~cm}$
- 網格大小： $0.33 \times 0.33 \times 0.25 \mathrm{~cm}$
- 楔形體： $4.8 \times 4.9 \times 2.4 \mathrm{~cm}$
- 變動條件：渠槽大小

Floating and sinking balls


Fast moving ball with collapsing water


## Rotation

$$
\omega_{y}=100(\mathrm{rad} / \mathrm{s})
$$

Cutplane W
7.1
$-1 e-05$
$-9 e-06$
$-8 e-06$
$-7 e-06$
$-6 e-06$
$-5 e-06$
$-4 e-06$
$-3 e-06$
$-2 e-06$
$-1 e-06$
-0

| 0.1 |
| :--- |



## Dam-break bore interacting with a movable ball



Water density 1000 kg/m ${ }^{3}$
Ball density 1200 kg/m ${ }^{3}$

Resolution $70 \times 30 \times 30$; Computational time: 3hrs 19min; CPU:i5-2500 CPU @ 3.30GHZ




## Two-way Coupled Moving Solid Method

 Implicit Velocity-Pressure Coupling (IVPC)

## Model Desoription

## Shape Description of An Egg-Shaped

 Particles
$C_{2}\left(x_{2}, y_{2}\right)$

Model Description
Shape Description of An Egg-Shaped Particles


## Model Description

## Four Kinds of Shape

$$
\begin{aligned}
& \mathrm{L} 1=0.0 \\
& \mathrm{R} 1=0.25 \\
& \mathrm{~L} 3=0.0 \\
& \mathrm{R} 3=0.25 \\
& \beta=60^{\circ}
\end{aligned}
$$


$\mathrm{L} 1=0.1$
$\mathrm{R} 1=0.1$
$\mathrm{~L} 3=0.2$
$\mathrm{R} 3=0.22$
$\beta=60^{\circ}$


## Model Description

Fluid-Structure Interaction Method

- Discrete Element Method (DEM)


Surface Force $=$ (Normal Stress + Shear Stres Area

$$
\tau=\mu \frac{\partial u}{\partial x}
$$

# Model Description <br> Fluid-Structure Interaction Method 

Calculation model of normal contact force


$$
F_{\text {total }}=F_{\text {surface }}+F_{\text {contact }}+F_{\text {Body }}
$$

## Model Validation Kármán Vortex Street Case




Moving Solid Algorithm
Partial Cell Treatment

## Model Validation <br> Kármán Vortex Street Case




Moving Solid Algorithm
Partial Cell Treatment

## Model Validation Water Entry Sphere

$$
\rho_{\mathrm{s}}=0.86\left(\mathrm{~g} / \mathrm{cm}^{3}\right)
$$





Case 1
Projectile Motion

$$
V_{0}=10 \mathrm{~m} / \mathrm{s}
$$



Case 1
Projectile Motion

$$
V_{0}=10 \mathrm{~m} / \mathrm{s}
$$



## Case 2 <br> Wave Impact and Solid Motion



## Case 2 <br> Wave Impact and Solid Motion

## Case 3

Wave Impact and Solid Floating


## Case 3 <br> Wave Impact and Solid Floating

## Case 4

## Wave Impact and Solid Rotation



## Case 4 <br> Wave Impact and Solid Rotation



## Case 5

## Wave Impact and Solid Rolling



## Case 5

Wave Impact and Solid Rolling


## Case 6 <br> Bouncing, Rolling, Floating

1.016967


## Case 6 <br> Bouncing, Rolling, Floating



## Case 7 <br> Projectile Motion and Collision



# Case 7 (2D-view) <br> Projectile Motion and Collision 

## Case 7 (3D-view) <br> Projectile Motion and Collision



## Results and Discussion Simulation Setup



## Run-Up Test

5 Meter Bore


## Run-Up Test 10 Meter Bore



## Run-Up Test <br> 15 Meter Bore



## Heat Conduction Test Case 3 Inflow Water Icing with Steel Pillar



## Heat Conduction Test Case 3 <br> Inflow Water Icing with Steel Pillar

## Heat Conduction Test Case 4 Dambreak Water Icing with Steel Pillar

Top Boundary , Pressure $=0$
Inflow_Material = Air


Steel Pillar :
Initial Temperature $=243 \mathrm{~K}$

## Heat Conduction Test Case 4 Dambreak Water Icing with Steel Pillar



## Heat Conduction Test Case 5

Splash Water Caused by Projectile and Icing with Ship


## Exhibit Case 1

Purpose : Simulate the stability of long-shape floating object when rockfall


## Exhibit Case 1

Purpose : Simulate the stability of long-shape floating object when rockfall


## Exhibit Case 2

Purpose : Simulate the stability of long-shape floating object when mudflow


## Exhibit Case 2

Purpose : Simulate the stability of long-shape floating object when mudflow


## Parameter of

## Exhibit Case 3 - Atatacels sata,000 Resolution $\mathrm{dx}=0.5 \mathrm{~m}$

- Sea Water Density $=1025.0 \mathrm{Kg} / \mathrm{m}^{3}$
- Water Heat Capacity = $3900.0 \mathrm{~J} /\left(\mathrm{k}_{\varepsilon}\right.$
- Water Initial Temperature = 278.0
- Sea Ice Density = $915.0 \mathrm{Kg} / \mathrm{m}^{3}$
- Ice Heat Capacity $=2000.0 \mathrm{~J} /(\mathrm{kg} . \mathrm{K})$
- Latent Heat = $334000.0 \mathrm{~J} / \mathrm{kg}$
- Ship Material : Steel Density $=7870.0 \mathrm{Kg} / \mathrm{m}^{3}$
- Steel Heat Capacity $=450.0 \mathrm{~J} /(\mathrm{kg} . \mathrm{K})$
- Steel Initial Temperature $=223.0 \mathrm{~K}$


## Exhibit Cas



Purpose : Simulate the icing situation of low-temperature ship with splash water caused by a sphere

## Bingham Constitutive Model Shear stress

$$
\begin{aligned}
& \mu(\bar{D})= \begin{cases}\text { ear stress } \\
\mu_{B}+\frac{\tau_{0}}{\sqrt{\frac{1}{2} \bar{D}: \bar{D}}} & \text { if } \frac{1}{2} \tau: \tau>\tau_{0}^{2} \\
\text { strain rate } \\
\text { Bingham viscosity }\end{cases} \\
& \begin{array}{ll}
(3) \\
\mu_{\infty} \\
\text { and } \bar{D}=0 & \text { if } \frac{1}{2} \tau: \tau \leq \tau_{0}^{2}
\end{array} \\
& \bar{D}=\dot{\gamma}_{i j}=\frac{\partial \dot{u}_{i}}{\partial x_{j}}+\frac{\partial \dot{u}_{j}}{\partial x_{i}} \\
& \text { A large number indicating the solid behavior (Bingham yield) } \\
& \begin{array}{l}
\text { There are three unknown variables onl } \\
\mu_{\infty} \text { is just a huge value to keep the rigidity }
\end{array}
\end{aligned}
$$

## 1. Pressure Gradient Channel Flow (Bird et al. 1983)

Newtonian Fluid


FIG. 2. Comparison between Computed and Analytical Velocity Profiles for Newtonian Fluid


Analytical Solution of Bingham Fluid in a Channel

$$
\begin{array}{ll}
u(y)=\frac{\left(P_{0}-P_{L}\right) B^{2}}{2 \mu_{B} L}\left[1-\left(\frac{y}{B}\right)^{2}\right]-\frac{\tau_{0} B}{\mu_{B}}\left(1-\frac{y}{B}\right) & y_{0} \leq y \leq B \\
u(y)=u\left(y_{n}\right)=u_{M} & \text { (Accurate turning point) }
\end{array} \begin{array}{ll}
0 \leq y \leq y_{n}
\end{array}
$$

## 2．Spreading of Bingham fluid on an inclined plane

Liu and Mei（1989）推導出斜板上之賓漢流理論解

## Experiment settings

Length ： 332 cm Width ： 7.62 cm Height： 15.24 cm
$\theta: 1.47^{\circ}$
Material ：Kaolinite mixed with tap water
$\rho: 1.106 \mathrm{~g} / \mathrm{cm}^{3}$
$\tau_{0}: 0.875 \mathrm{~Pa}$
$\mu: 0.034 P a \cdot S$


## Spreading of Bingham fluid on an inclined plane

## Numeric settings

Domain : $200.0 \mathrm{~cm}{ }^{*} 0.05 \mathrm{~cm} * 1.0 \mathrm{~cm}$ Cells : 4000 * 1 * 20 $\theta: 1.47^{\circ}$
$d x=d y=d z=0.05 \mathrm{~cm}$
$\rho: 1.106 \mathrm{~g} / \mathrm{cm}^{3}$
$\tau_{0}: 0.875 \mathrm{~Pa}$
$\mu: 0.034 \mathrm{~Pa} \cdot \mathrm{~S}$
$\mu_{\text {inf }}: 10^{10} \mathrm{~Pa} \cdot S$



## Schematic of Discontinuous Bi-viscous Model (DBM)


－Loose structure without tamping：
Angle＝Angle of repose
－未夯實，結構鬆散：
角度為安息角

－Tight structure after tamping or settlement： Angle＞Angle of repose
－夯實後，結構緊實，角度大於安息角

（Chinatimes）

## Discontinuous Bi-viscous Model (DBM) <br> Equations of Rheology

$$
\begin{aligned}
& \tau=\mu(\bar{D}) \bar{D} \\
& \mu(\bar{D})= \begin{cases}\mu_{\infty} & \text {, if } \bar{D}>\bar{D}_{y} \\
\mu_{B}+\frac{\tau_{0}}{\sqrt{\frac{1}{2} \bar{D}: \bar{D}}} \text {, if } \bar{D} \leq \bar{D}_{y}\end{cases} \\
& \bar{D}=\dot{\gamma}_{i j}=\frac{\partial \dot{u}_{i}}{\partial x_{j}}+\frac{\partial \dot{u}_{j}}{\partial x_{i}}
\end{aligned}
$$

Only 4 unknown variables: $\bar{D}_{y} \mu_{\infty} \mu_{B} \tau_{0}$

## BM vs DBM

Bi-viscous Model (BM)


Discontinuous Bi-viscous Model (DBM)


The development of Slip surface can be seen clearly

Simulation on the Sand Sliding Down by DBM


## 3. Failure of Gypsum Tailings Dam East Texas, 1966

Jeyapalan (1983)
Initial Height of Dam : 11 m
Material : Gypsum Tailings
Bed Slope : $0^{\circ}$
Properties of Tailings :

$$
\begin{aligned}
& \rho=1400.0 \mathrm{~kg} / \mathrm{m}^{3} \\
& \tau_{0}=1000.0 \mathrm{~Pa} \\
& \mu=50.0 \mathrm{~Pa} \cdot S
\end{aligned}
$$

Mu_max=1.e6, ss_c=0.2


Flow of Liquefied Tailings from Gypsum Tailings Impoundment (1966)


Elevation ( $\mathrm{t}=0 \sim 200 \mathrm{~s}$ )
0
Isovolume Vect Mag


Velocity Magnitude ( $\mathrm{t}=0 \sim 200 \mathrm{~s}$ )


Flow surface after freezing time computed by Splash3D model 109

## Result Competition

|  | Inundation distance <br> $(\mathrm{m})$ | Freezing time <br> $(\mathrm{s})$ | Mean velocity <br> $(\mathrm{m} / \mathrm{s})$ |
| :---: | :---: | :---: | :---: |
| Observed values | 300 | $60-120$ | $2.5-5.0$ |
| Theoretical results from <br> charts | 550 | 132 | 4.2 |
| Result using TFLow <br> (Jeyapalan, 1983 ) | 470 | 85 | 5.5 |
| Result computed by <br> Pastor et al. (2004) | 170 | 120 | 1.4 |
| Result computed by <br> Chen (2006) | 200 | 120 | 1.7 |
| Result using Splash3D <br> model | 310 | 130 | 2.4 |

## 5. Simulation on the failure of Shuan-Yuan Bridge

in the event of 2009 Typhoon Morakot
The undular waves imply the uneven soft bottom

基礎沖刷
波狀水躍的發生，通常意味底泥鬆軟：基椿之局部沖刷嚴重


## 3D Local scour induced by the strong flood

mud_vof $=0.05$


## Comparison to the Field Survey Data

## Maximum Scour Depth：

Right in front of the bridge piers：
Field survey：about 23 m ．
Numerical： 23 m ．

30 m upstream away from the bridge piers：
Field survey： 15 m
Numerical： 15 m

地電阻法
（Electrical Resistivity Tomography，ERT）

2e31．60＋66e30．60＝2082．80



## Dirty harbor in Japan?



